

Similarly, from Fig. 7.5.4, the column forces, the shears, and the moments in the interior columns become:

$$V_4 - V'_4 + R_4 = 0 \quad \therefore V'_4 = \frac{40}{4} \left( \frac{l}{h} \right) W,$$

$$V_3 - V'_3 + R_3 - R_4 = 0 \quad \therefore V'_3 = \frac{40}{16} \left( \frac{l}{h} \right) W,$$

$$V_2 - V'_2 + R_2 - R_3 = 0 \quad \therefore V'_2 = \frac{40}{32} \left( \frac{l}{h} \right) W,$$

$$V_1 - V'_1 + R_1 - R_2 = 0 \quad \therefore V'_1 = \frac{40}{48} \left( \frac{l}{h} \right) W,$$

$$H''_4 \frac{2}{h} - V'_4 \frac{2}{l} - V'_4 \frac{2}{l} = 0 \quad \therefore H''_4 = \frac{40}{7} W,$$

$$H''_3 \frac{2}{h} + H''_4 \frac{2}{h} - V'_3 \frac{2}{l} - V'_3 \frac{2}{l} = 0 \quad \therefore H''_3 = \frac{40}{21} W,$$

$$H''_2 \frac{2}{h} + H''_3 \frac{2}{h} - V'_2 \frac{2}{l} - V'_2 \frac{2}{l} = 0 \quad \therefore H''_2 = \frac{40}{35} W,$$

$$H''_1 \frac{2}{h} + H''_2 \frac{2}{h} - V'_1 \frac{2}{l} - V'_1 \frac{2}{l} = 0 \quad \therefore H''_1 = \frac{50}{49} W; \quad H''_0 = H''_1,$$

$$M_4 = V_4 \frac{l}{3} = \frac{80}{3} Wh, \quad M'_4 = H''_4 \frac{2}{7} = \frac{80}{7} Wh,$$

$$M_3 = V_3 \frac{l}{12} = \frac{80}{12} Wh, \quad M''_3 = H''_3 \frac{2}{7} = \frac{80}{7} Wh, \quad M'_3 = H''_3 \frac{2}{21} = \frac{80}{21} Wh,$$

$$M_2 = V_2 \frac{l}{24} = \frac{80}{24} Wh, \quad M''_2 = H''_2 \frac{2}{21} = \frac{80}{21} Wh, \quad M'_2 = H''_2 \frac{2}{35} = \frac{80}{35} Wh,$$

$$M_1 = V_1 \frac{l}{36} = \frac{80}{36} Wh, \quad M''_1 = H''_1 \frac{2}{35} = \frac{80}{35} Wh, \quad M'_1 = H''_1 \frac{2}{49} = \frac{80}{49} Wh,$$

$$M''_3 = V'_3 \frac{l}{16} = \frac{80}{16} Wh,$$

$$M''_2 = V'_2 \frac{l}{32} = \frac{80}{32} Wh,$$

$$M''_1 = V'_1 \frac{l}{48} = \frac{80}{48} Wh,$$

$$M''_0 = H''_0 \frac{2}{49} = \frac{80}{49} Wh.$$

**Example 7.5.1.** Determine the vertical load moments in the frame of Fig. 7.5.5, assuming the frame to be one of a set 6 m (20 ft) on centers with a total uniform load of 10 kN/m<sup>2</sup> (200 psf).