

1) **IS 1893-2002**

P-Cube Consultants

Design Acceleration spectrum = $Z/2 \cdot I/R \cdot Sa/g$ cl 6.4

where $I =$

1
0.16
5
1

 SMRF
 $Z =$

0.16

 $R =$

5

 SMRF
 $T =$

1

 sec
 Soil Medium type
 $sa/g = 1.36$

$A_h = 0.02176$

2) **IS 1893-2016**

Design Acceleration spectrum = $Z/2 \cdot I/R \cdot Sa/g$ cl 6.4

where $I =$

1.2
0.16
5
1

 for more than 200 people occupancy
 $Z =$

0.16

 $R =$

5

 SMRF
 $T =$

1

 sec
 Soil Medium type
 $sa/g = 1.36$

$A_h = 0.026112$

3) **IS 1893 Draft 2023**

Design Acceleration spectrum = $Z \cdot I \cdot A_h(T)/R$

where $I =$

1
0.2
5
1

 Building set-2
 $Z =$

0.2

 975 Return periods more than 200 person
 $R =$

5

 SMRF
 $T =$

1

 sec
 Soil Site Class C where
 here $V_s = E_{hi}/E_{(hi/vsi)}$ (Isolated footing)
 $v_{si} = 80 (N_i)^{0.5}$ Cohessionless
 $v_{s1} = 438.178$ $v_{s2} = 438.17805$ $v_{s3} = 473.2864$
 N value approx. 30 for two layer of 300mm and 35 for one layer 300mm
 $V_s = 449.2874$ m/s

$A_h = 1.5$
 $A_h(T) = 1.5$ for 5% damping
 $A_h(T) = Z \cdot I \cdot A_h(T)$
 $A_h(T) = 0.3$
 $A_hs(T) = 0.06$

4) **NZS 1170.5:2004** Newzeland

Elastic site spectrum horizontal loading $C(T)=Ch(T)*Z*R*N(T,D)$

Design Acceleration spectrum $Cd(T)= C(T)*Sp/ku$

$$Ch(T) = \begin{matrix} 1.19 \\ 0.16 \end{matrix} \begin{matrix} \text{for } T=1 \text{ and shallow soil Velocity more than 300} \\ \text{Hamilton to wellington more than 300km} \\ \text{(like Ahmedabad to Bhuj)} \end{matrix}$$

$$Z = \begin{matrix} 1.3 \\ 1 \\ 1 \end{matrix} \begin{matrix} \text{Return period factor 1000 year} \\ \text{sec} \\ \text{annual probability } > 1/250 \end{matrix}$$

$$C(T) = 0.24752$$

Here $Sp = \begin{matrix} 0.7 \\ 3 \\ 6 \end{matrix}$

$Ku = \begin{matrix} 3 \\ 6 \end{matrix}$ SLD (Structure limited ductility) = 1.25 to 3

$Ku = \begin{matrix} 6 \end{matrix}$ DS (Ductile system) = 1.25 to 6 (max ductile material or any system)

$$Cd(T) = 0.057755 \text{ SLD}$$

$$Cd(T) = 0.028877 \text{ DS}$$

$$\text{Min } Cd(T) = (Z/20+0.02)R \text{ but min } 0.03R$$

$$\text{Min } Cd(T) = 0.0364 \text{ but min } 0.039$$

SLD $Cd(T) = 0.057755$

DS $Cd(T) = 0.039$

5) **ASCE 7-16** U.S.

Seismic Response coefficient $Cs = Sds/(R/I)$

where $I = \begin{matrix} 1.25 \\ 5 \\ 1 \end{matrix}$ $\begin{matrix} \text{Category 3 of important structure} \\ \text{B type line4 Special Reinforce concrete shear wall} \\ \text{sec} \end{matrix}$

$$Sds = 2/3 * Sms$$

$$Sms = Fa * Ss$$

$$Sdl = 2/3 * Sml$$

$$Sml = Fv * Sl$$

Class C 366m/s to 446 m/s

22-1,22-2 Los angles to 300km or Memphis to 300km

$$Fa = 1.2 \text{ Table 11.4.1}$$

$$Fv = 1.5 \text{ Table 11.4.2}$$

$$Ss = 0.3 \text{ Memphis to 300km}$$

$$Sl = 0.15 \text{ Memphis to 300km}$$

$$Sds = 0.24$$

$$Sdl = 0.15$$

$$TL = 12$$

$$T < TL \quad Cs = Sdi/T * I/R = 0.0375$$

$C_s = 0.06$ minimum $C_s = 0.044 * S_d s * I$ but min. 0.01
 min. $C_s = 0.0132$
 if $S_I > 0.6$ $C_s = 0.5 S_I / (R/I)$
 $C_s = 0.01875$

C_s for 0.2 sec 0.06
 C_s for 1 sec 0.0375

6) EUROCODE 8-2004 Singapore

Horizontal Base shear $F_b = S_d(T) * W * y$

Ground Type **Class C** for N value 15 to 50 No.

According to Table 3.2 & 3.3

	TYP-1	TYP-2
S=	1.15	1.5
T _b =	0.2	0.1
T _c =	0.6	0.25
T _d =	2	1.2

T= **1** sec

$T_c < T < T_d$

$S_d(T) = a_g * S * 2.5 / q * (T_c / T)$

> $\beta * a_g$

$q_0 =$ **4.5** for DCH System

$q = q_0 * (\alpha_{hau} / \alpha_{h1}) * k_w$

$(\alpha_{hau} / \alpha_{h1})$ **1.3** multi storied & multi bays

$q = 5.85$

$\gamma =$ **1**

$a_g = \gamma * a_{gR}$

a_g $S_d(T)$ (Ty-1) $S_d(T)$ (Ty-2)

Singapore 475 return period single zone

0.175 0.052 0.028

Germany 975 year Stuttgart to 220km away Nuremburg

0.312 0.092 0.050

Comparisons Table

	Different codes						Singapore	
	IS 1893-2002	IS 1893-	IS 1893-	NZS 1170.5:2004		ASCE 7-16		Eurocode 8
	(After Bhuj EQ)	2016	2023 Draft	SLD	DS	Short T	Long T	
Ah or cd or cs	0.0218	0.0261	0.0600	0.0578	0.0390	0.0600	0.0375	0.052
Diff to IS 1893-2002	1.00	1.20	2.76	2.65	1.79	2.76	1.72	2.37
EQ Load factor	1.50	1.50	1.00	1.00	1.00	1.00	1.00	1.00
Diff to IS 1893-2002	1.00	1.20	1.84	1.77	1.19	1.84	1.15	1.58

****This all data only for understanding and information purpose

Design Earthquake Zone Factor Z

Earthquake Zone	Zone-2	Zone-3	Zone-4	Zone-5	Zone-6	
for different Return Periods TRP (years)	73	0.0375	0.075	0.18	0.24	0.3
	225	0.05	0.1	0.225	0.3	0.375
	475	0.075	0.15	0.3	0.4	0.5
	975	0.1	0.2	0.36	0.48	0.6
	2,475	0.15	0.3	0.45	0.6	0.75
	4,975	0.2	0.4	0.54	0.75	0.9375
	9,975	0.25	0.5	0.675	0.9	1.125

During Bhuj 2001 Earthquake PGA in Ahmedabad at various site is 0.1g to 0.11g (in 0.005 sec time step)

1

PGA (max) single direction	PGA (Max) (vector sum h1, h2,v)	Magnitude	Depth	Earthquake
3.23 g		7.8	15 km	2016 Kaikoura earthquake
2.7 g	2.99 g	9.1	30 km	2011 Tohoku earthquake and tsunami
	4.36 g	6.9	8 km	2008 Iwate–Miyagi Nairiku earthquake
1.92 g		7.7	8 km	1999 Jiji earthquake
1.82 g		6.7	18 km	1994 Northridge earthquake
1.81 g		9.5	33 km	1960 Valdivia earthquake
1.62 g		7.8	10 km	2023 Turkey–Syria earthquake
1.51 g		6.2	5 km	2011 Christchurch earthquake
1.26 g		7.1	10 km	2010 Canterbury earthquake
1.25 g		6.6	8.4 km	1971 Sylmar earthquake
1.04 g		6.6	10 km	2007 Chuetsu offshore earthquake
0.98 g		7	16.1 km	2020 Aegean Sea earthquake
0.91 g		6.9	17.6 km	1995 Great Hanshin earthquake
0.8 g		7.2	12 km	2013 Bohol earthquake
0.65g		6.9	19 km	1989 Loma Prieta earthquake
0.5 g		7	13 km	2010 Haiti earthquake
0.34 g		6.4	15 km	2006 Yogyakarta earthquake
0.18 g		9.2	25 km	1964 Alaska earthquake